

SIDDARTHA INSTITUTE OF SCIENCE AND TECHNOLOGY:: PUTTUR (AUTONOMOUS)

Siddharth Nagar, Narayanavanam Road – 517583

OUESTION BANK (DESCRIPTIVE)

Subject with Code: Mechanics of Solids (18CE0111)

Course & Branch: B.Tech – CE

Year & Semester: II-B.Tech & II-Semester

Regulation: R18

<u>UNIT 1</u>

THIN & THICK CYLINDERS

- 1. A cylindrical thin drum 80 cm in diameter and 3 m long has a shell thickness of 1 cm. If the drum is subjected to an internal pressure of 2.5 N/mm², Determine (i) change in diameter (ii) change in length and (iii) change in volume. Take E= 2x 10 5 N/mm² Poisson's ratio 0.25. [10M]
- 2. A cylindrical shell 100 mm long 200mm internal diameter having thickness of a metal as 10 mm is filled with a fluid at atmospheric pressure. If an additional 200 mm³ pumped into the cylinder, Find i) the pressure exerted by the fluid on the cylinder and ii) the hoop stress induced. Take $E = 2 \times 10^{5} \text{ N/ mm}^{2}$ and Poisson's ratio is 0.3 [10M]
- 3. A copper cylinder, 90 cm long, 40 cm external diameter and wall thickness 6 mm has its both ends closed by rigid blank flanges. It is initially full of oil at atmospheric pressure. Calculate additional volume of oil which must be pumped into it in order to raise the oil pressure to 5 N/mm 2 above atmospheric pressure. For copper assume $E= 1.0 \times 105 \text{ N/mm 2}$ and Poisson's ratio 1/3. Take bulk modulus of oil as $K= 2.6 \times 103 \text{ N/mm}^2$. [10M]
- 4. A closed cylindrical vessel made of steel plates 4 mm thick with plane and, carries fluid under a pressure of 3 N/ mm². The dia, of cylinder is 30 cm and length is 80 cm, calculate the longitudinal and hoop stresses in the cylinder wall and determine the change in diameter, length and volume of the cylinder. Take $E = 2 \times 10^{5} \text{ N/ mm}^{2}$ and Poisson's ratio is 0.286 [10M]
- 5. Derive an expression for hoop and radial stresses across thickness of the thick cylinder [10M]
- Calculate the thickness of metal necessary for a cylindrical shell of internal diameter 160 mm to withstand an internal pressure of 8 N/mm 2, if maximum hoop stress in the section is not exceed to 35N/mm².
- 7. Determine the maximum and minimum hoop stress across the section of a pipe of 400 mm internal diameter and 100 mm thick, when the pipe contains a fluid at a pressure of 8 N/mm 2. Also sketch the radial pressure and hoop stress distribution across the section. [10M]
- 8. A compound cylinder is made by shrinking a cylinder of external diameter 300 mm and internal diameter of 250 mm over another cylinder of external diameter 250 mm and internal diameter 200 mm. The radial pressure at the junction after shrinking is 8 N/mm². Find the final stresses set up across the section, when the compound cylinder is subjected to an internal fluid pressure of 84.5 N/mm².

- 9. A steel cylinder of 300 mm external diameter is to be shrunk to another steel cylinder of 150 mm internal diameter. After shrinking, the diameter at the junction is 250 mm and radial pressure at the common junction is 28 N/mm². Find the original difference in radii at the junction. Take $E = 2 \times 105 \text{ N/mm}^2$. [10M]
- 10. A thick spherical shell of 200 mm internal diameter is subjected to an internal fluid pressure of 7 N/mm². If the permissible tensile stress in the shell material is 8 N/mm², Find thickness of the shell. [10M]

<u>UNIT-II</u>

DIRECT AND BENDING STRESS

- 1. A masonry dam of rectangular section, 20 m high and 10 m wide, has water upto a height of 16 mon its one side find:
 - i) Pressure force due to water on one meter length of the dam
 - ii) Position of centre of pressure
 - iii) The position at which the resultant cuts the base and Maximum and minimum intensities at the base of the dam. Take weight density of masonry is 19.62 kN/m^3 and of water 9.81 kN/m^3
 - [10M]
- 2. Derive kernel of section for Rectangular, Circular and Hallow Circular sections [10M]
- 3. a) Derive the equation for resultant stresses when a column of rectangular section is subjected to a load which is eccentric to both axes [7M]

b) A short column of rectangular cross-section 80 mm by 60 mm carries a load of 40 KN at a point 20 mm from the longer side and 35 mm from the shorter side. Determine the maximum compressive and tensile stresses in the section.

[5M]

- 4. Determine the maximum and minimum stresses at the base of a hollow circular chimney of height 20m with external diameter 4m. The chimney is subjected to a horizontal wind pressure of intensity 1 KN/m². The specific weight of the material of chimney is 22 KN/m³ [10M]
- 5. Find the position of centroid I_{XX} and I_{YY} for an unequal angle section 125mm X 75mm X 10mm [10M]

UNSYMMETRICAL BENDING

- 6. A cantilever of length 2m carries a point load of 2KN at the free end. The c/s of cantilever is an unequal of dimensions 150X50X15 mm³. The small leg of angle 50 mm is horizontal. The load passes through the centroid of the c/s. Determine a) position of neutral axis b) the magnitude of maximum stress setup at the fixed section of the cantilever [10M]
- A 45 mm X 45 mm X 5 mm angle is used as a SSB over a span of 2.4m. It carries a load of 300 N along the vertical axis passing through the centroid of the section. Determine the resulting bending stress on the outer corners of the section, along the middle section of the beam [10M]
- 8. Determine the centroidal moment of inertia of the equal section $30 \times 30 \times 10 \text{ mm}^3$ [10M]
- 9. a) What is unsymmetrical bending

b) Determine the prinicipal moment of inertia of unequal angle section 200X 150 X 10 X mm³ [10M]

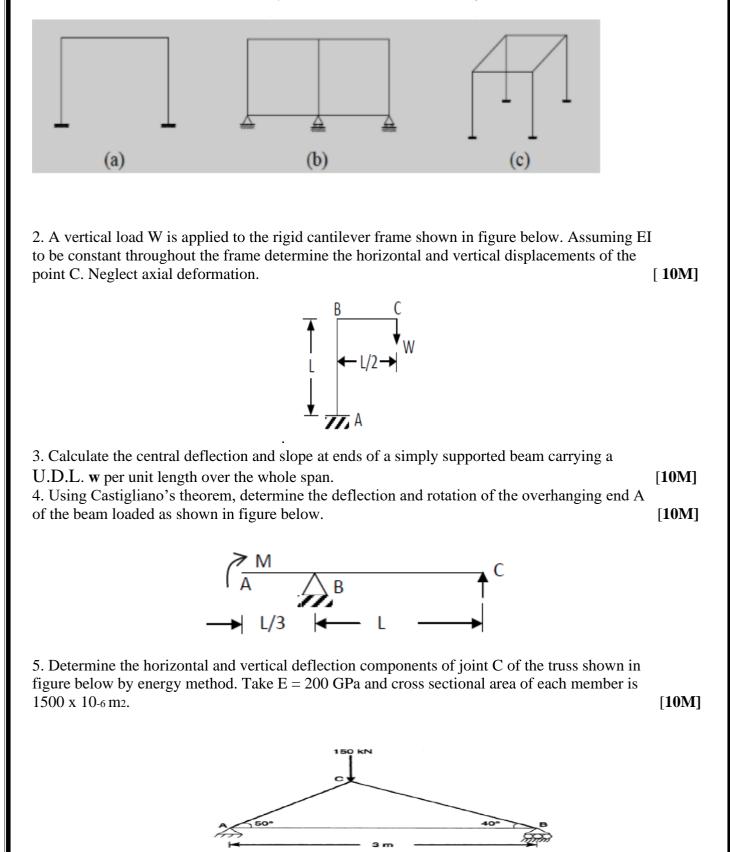
10. A wooden beam of c/s 100 mm X 150 mm is used as shown in fig to support a sloping tiled roof. It has an effective span of 4m and carries a uniformly distributed load 3 KN/M acting vertically download. Determine the maximum stresses developed in the beam [10M]

Mechanics of solids (18CE0111)

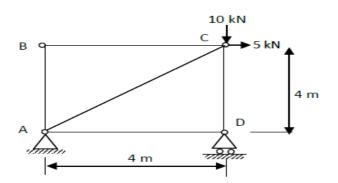
[2M]

UNIT-III INTROUDUCTION

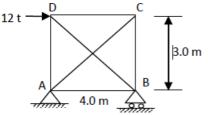
1) Determine the number of indeterminacy for the three frames shown in figure below.



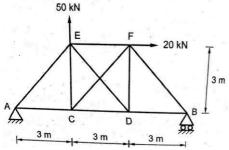
6. Find horizontal and vertical deflection of joint C of truss ABCD loaded as shown in figure below. Assume that, all members have the same axial rigidity.



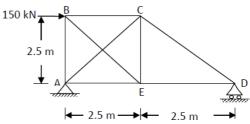
7. A pin jointed framed structure is loaded as shown in figure below. Calculate the forces in all members. Take area for horizontal members as 20 cm^2 , vertical members as 30 cm^2 , inclined members as 50 cm^2 and $\text{E} = 2000 \text{ t/cm}^2$. [10M]



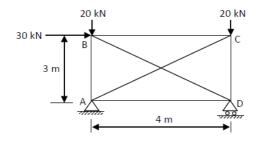
8. Analyze the truss shown in figure below. Assume that the cross sectional area of all members are same. [10M]



9. Determine the force in the members AC of a pin-jointed truss shown in figure below. Assume crosssectional area of each member to be $15 \times 10^{-4} \text{ m}^2$. [10M]



10.Determine the stresses in all the members of the frame shown in figure below, in which the cross sectional area of vertical members are 30 cm² each and those of all other members are 22 cm². Take E = 200 GPa. [10M]



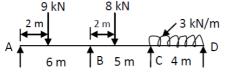
Mechanics of solids (18CE0111)

[10M]

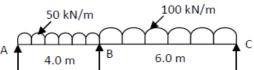
UNIT-IV

ANALYSIS OF FIXED BEAMS & CONTINUOUS BEAMS

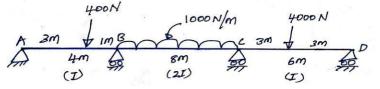
- 1. A fixed beam of length 6 m carries two point loads of 30 kN each at a distance of 2 m from both ends. Determine the fixed end moments and draw BMD. [10M]
- 2. A Fixed beam of span 6 m is subjected a UDL of 5 kN/m on the left half of the span and a point load of 15 kN at the middle of the right half of the span. Draw the SFD and BMD [10M]
- Calculate the fixed end moments and the reactions at the supports for a fixed beam AB of length 6 m. The beam carries point loads of 160 KN and 120 KN at a distance of 2 m and 4 m from the left end A. Draw SFD & BMD.
- 4. Derive an expression to find BM and SF of fixed beam carrying an eccentric load. [10M]
- Determine the fixed end moments for the fixed beam with applied clockwise moment 'M' of distance 'a' from left end. The total length of beam is 'L'. Sketch the bending moment and shear force diagram. [10M]
- 6. A continuous beam ABC of constant moment of Inertia carries a load of 10 kN in mid span AB and a central clockwise moment of 30 kN-min span BC. Span AB = 10 m and span BC = 15 m. Find the support moments and plot the shear froce and bending moment diagram. [10M]
- Analyze the continuous beam ABCD shown in the figure below using theorem of three moments. Draw SFD and BMD. [10M]



- 8. A continuous beam ABC of uniform section with span AB and BC as 4 m each, is fixed at A and simply supported at B and C. The beam is carrying a uniformly distributed load of 6 kN/m run throughout its length. Find the support moments and the reactions using theorem of three moments. Also draw SFD and BMD. [10M]
- 9. Analyze the beam and draw BMD and SFD



10. A continuous beam ABCD 18 m long is loaded as shown in figure below. During loading support 'B' sinks by 10 mm Find support moments and plot shear force and bending moment diagrams for the beam. Take E= 20 kN/mm², I = 8 X 10⁶ mm⁴
[10M]



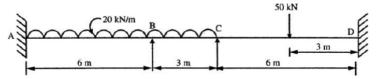
[10M]

10M

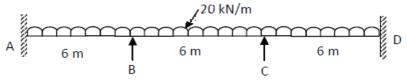
<u>UNIT-V</u>

SLOPE DEFLECTION METHOD& MOMENT DISTRIBUTION METHOD

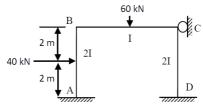
1. Analyze the continuous beam as shown in figure below by slope deflection method. Support B sinks by 10 mm. Take E = 200 GPa and $I = 16 \times 10^7$ mm⁴. Draw the bending moment diagram.



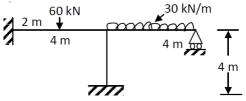
 Analyze the continuous beam shown in figure below by slope deflection method and sketch SFD and BMD. EI is constant.
 10M



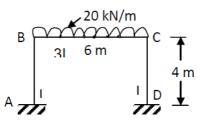
Analyze the portal frame shown in figure below, by slope deflection method. The relative moment of inertia value for each member is indicated in the figure below. Sketch the bending moment diagram
 10M



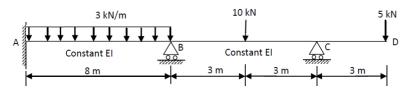
4. Analyze the frame shown in figure by slope deflection method. Draw BMD flexural rigidity is same for all members 10M



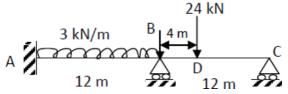
5. Analyze the frame shown in figure by slope deflection method. Draw BMD flexural rigidity is same for all members 10M



6.Analyze the continuous beam shown in figure below, using moment distribution method. Draw shear force and bending moment diagram for the continuous beam.10M

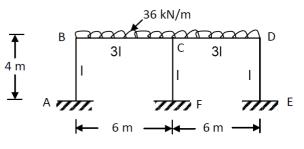


7.Analyze the continuous beam shown in figure below by using moment distribution method. The support B sinks 30 mm, values of E and I are 200 GPa and 0.2 x 10⁹ m⁴ respectively uniform throughout. Draw S.F and B.M diagrams.
 10M



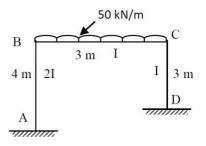
8. Analyze the rigid jointed frame shown in figure by moment distribution method and draw BMD





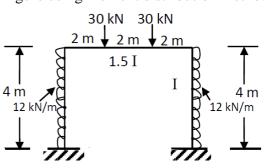
9. Analyze the portal frame shown in figure using moment distribution method





10. Analyze the portal frame shown in figure using moment distribution method

10M



SIDDHARTH INSTITUTE OF ENGINEERING & TECHNOLOGY:: PUTTUR (AUTONOMOUS) Siddharth Nagar, Narayanavanam Road - 517583 **OUESTION BANK (OBJECTIVE) Course & Branch**: B.Tech – CE Subject with Code : Mechanics of Solids -I(18CE0111 Year & Semester: II-B.Tech & I-Semester **Regulation:** R18 **UNIT-I** THIN &; THICK CYLINDERS 1) A cylindrical vessel is said to be thin if the ratio of its internal diameter to the wall thickness is A) < 20 B) > 20C) = 20 D) None ſ 1 2) A cylindrical vessel is said to be thick if the ratio of its internal diameter to the wall thickness is **B**) > 20 A) < 20 C) = 20D) None Γ 1 3) The hoop stress in a thin cylindrical shell is 1 [B) PD/4tA) PD/2tC) 6PD/tD) 4PD/t4) The longitudinal stress in a thin cylindrical shell is] B) PD/4tA) PD/2tC) 6PD/tD) 4PD/t5) Circumferential and hoop stress in thin spherical shell are] ſ B) Not Equal A) Equal C) Greater D) Lesser 6) The maximum shear stress in a thin cylindrical shell is 1 Γ A) F/AB) A/F C) FA D) None 7) A cast iron pipe 1m diameter is required to withstand a 200m head of water. If the tensile stress is 20Mpa, then the thickness is 1 A) 25mm B) 50mm C) 75mm D) 100mm 8) For the analysis of thick cylinders, the theory applicable is 1 ſ A) Lame's theory B) Rankine's theory C) Poisson's theory D) None 9) The variation of the hoop stress across the thickness of a thick cylinder is A) Linear B) Parabolic C) Uniform D) hyperbolic [1 10) A thin cylindrical shell of internal dia D, and thickness t, is subjected to internal pressure p. The change in diameter is given A) $Pd2/4tE(2-\mu)$ B) $Pd2/4tE(1-2\mu)$ C) Pd2/2tE (2- μ) D) Pd2/2tE $(1-2\mu)$ 11) The ratio of circumferential stress to longitudinal stress in z thin cylinder subjected to an internal Hydrostatic pressure is 1 A) 1/2 **B**) 1 C) 2 D) 4 Mechanics of solids (18CE0111) Page 9

12) A thick cylinder is	5	1	1 1	ss on th	e outer
surface is 150Mpa, A) 105Mpa	then the hoop stress (B) 180 Mpa	C) 210 Mpa	D) 135Mpa	L]
	cal steel pressure vess ssure if intensity p. If				
A) 18Kg/cm ²	B) 36Kg/cm ²	C) 180Kg/cm ²	D) 360Kg/cm	n^2]
of shell is 0.3. Due t	al shell is subjected to to internal pressure th circumferential to axis	ne shell is subjected			
A) 0.425	B) 2.25	C) 0.225	D) 4.25	L	1
	ternal dia, wall thickn thin spherical vessel		nternal pressure, the	ratio of ۲	maximum
A) 2	B) ¹ ⁄2	C) 4	D) ¼	L	1
0.6m and the tensile minimum thickness		l is to be limited to	9000 N/mm 2 . The		
A) 9 mm	B) 11 mm	C) 17mm	D) 21mm		
	ler pressurized from i all thickness B) outer	· ·		[p; C]
-	r with internal diamet		a 2d is subjected to in	nternal	
Then the maximum A) P	hoop stress in the cyl B) 0	linder is C) 1	D) 2p	[]
	used technique of strength of Strength of Shrink Fitting C	0 0 1		[]
20) A thick cylinder A) Outer radius	r under external fluid s B) Inner radius	pressure' p 0' will C) Mean radius		ss at the [e]
· · ·	r under internal fluid s B) Inner radius	pressure' pi will ha C) Mean radiu		at the []
· •	r under pi and po will s B) Inner radius	have maximum str C) Mean radiu		[]
· · · ·	in thick cylinders is or resses B) Decreased s		1 stresses D) None	[]
24) Which stress is t A) Longitudina	the least in a thin she l stress B) Hoop st		lial stress D) N	[one]
· · · ·	ndrical and spherical	thin vessels of sam	e material, diameter	and pre	essure
which has the lesser A) Cylindrical C) Cylindrical			B) Spherical shell D) None	[]

and under same p A) 4:1	B) 2:1	C) 1	1:2	D) 1:4	
27) Stresses in a t A) Diameter	•		rnal pressure is ind Length	ependent of [D) Diameter an	d thickn
28) Design of a th A) Radial st	nin shell under p ress B) Longitu			[D) All the three] stresses
29) Which is mos A) Bearing f			a thin shell ilure C) Crushing	[failure D) None]
30) The maximur A) σ h/E +µσ		•	er pi will be C C) σ r/E +μσ l	[/E D) None] •
31) Stress in a beA) Directly pr			pportional C) Cu	[rvilinearly relate] d D) No
32) Which stress A) Shear stres		ere is an eccentr Bending stress	ic load applied? C) Tensile stre] ss D) Therr] nal stres
	ly proportional t		of layer from the ne	•]
C) Direct	ly proportional to	o the neutral lay	of layer from the n yer yer from the neutra	-	
C) Direct D) Does r	ly proportional to not depend on the of a beam, which	o the neutral lay e distance of lay	yer yer from the neutra ich is neither elong	l layer	
 C) Direct D) Does r 34) On bending of A) Axis of lo 35) Which of the 	ly proportional to not depend on the of a beam, which oad B) se are types of n and compressive	o the neutral lay e distance of lay n is the layer wh Neutral axis ormal stresses?	yer yer from the neutra ich is neither elong C) Cen B) Tensile and	l layer gated nor shorten ter of gravity D)) None]
 C) Directing D) Does response of the constraint of the constraint	ly proportional to not depend on the of a beam, which oad B) se are types of n and compressive d bending nich acts in a dire	o the neutral lay e distance of lay n is the layer wh Neutral axis ormal stresses? stresses	yer yer from the neutra ich is neither elong C) Cen B) Tensile and	I layer gated nor shorten ter of gravity [thermal stresses ye and plane stress called [)) None]
 C) Directing D) Does response of the constraint of the constraint	ly proportional to not depend on the of a beam, which oad B) se are types of n and compressive d bending nich acts in a dire ress B)	o the neutral lay e distance of lay n is the layer wh Neutral axis formal stresses? stresses ection perpendio Normal stress	yer yer from the neutra ich is neither elong C) Cen B) Tensile and D) Compressiv cular to the area is	l layer gated nor shorten ter of gravity [thermal stresses ye and plane stres called [ess d) None)) None] sses]
 C) Directing D) Does response of the constraint of the constraint	ly proportional to not depend on the of a beam, which oad B) se are types of n and compressive d bending nich acts in a dire ress B)	o the neutral lay e distance of lay n is the layer wh Neutral axis formal stresses? stresses ection perpendio Normal stress	yer yer from the neutra ich is neither elong C) Cen B) Tensile and D) Compressiv cular to the area is C) Thermal str	l layer gated nor shorten ter of gravity [thermal stresses ye and plane stres called [ess d) None)) None] sses]
 C) Directing D) Does r 34) On bending of A) Axis of loc 35) Which of the A) Tensile a C) Shear and 36) The stress where A) Shear stress 37) In a body load component A) 1 	ly proportional to not depend on the of a beam, which oad B) se are types of n and compressive d bending nich acts in a dire ress B) aded under plane nts? B) 2 of stress does in a	o the neutral lay e distance of lay n is the layer wh Neutral axis ormal stresses? stresses ection perpendio Normal stress e stress conditio C) 3	yer yer from the neutra ich is neither elong C) Cen B) Tensile and D) Compressiv cular to the area is C) Thermal str ns, what is the num D) 6 bar is taken by the	l layer gated nor shorten ter of gravity [thermal stresses 'e and plane stres called [ess d) None hber of independ [)) None] sses] ent stres]]
 C) Directing D) Does response of the constraint of the constraint	ly proportional tenot depend on the not depend on the of a beam, which oad B) I se are types of n and compressive d bending nich acts in a direct ress B) I aded under plane nts? B) 2 of stress does in a stress B) 0	o the neutral lay e distance of lay n is the layer wh Neutral axis ormal stresses? stresses ection perpendio Normal stress e stress conditio C) 3 a reinforcement Compressive st tio of 0.5. If uni	yer yer from the neutra ich is neither elong C) Cen B) Tensile and D) Compressiv cular to the area is C) Thermal str ns, what is the num D) 6 bar is taken by the ress C) Shear str iform pressure of 3	l layer gated nor shorten ter of gravity [thermal stresses re and plane stres called [ess d) None hber of independ [concrete? [ress d) Bend)) None] sses] ent stress] ing stress
 C) Directing D) Does response of the constraint of the constraint	ly proportional te not depend on the of a beam, which oad B) se are types of n and compressive d bending nich acts in a dire ress B) aded under plane nts? B) 2 of stress does in a stress B) as a Poisson's rat What will be the	o the neutral lay e distance of lay n is the layer wh Neutral axis ormal stresses? stresses ection perpendio Normal stress e stress conditio C) 3 a reinforcement Compressive st tio of 0.5. If uni	yer yer from the neutra ich is neither elong C) Cen B) Tensile and D) Compressiv cular to the area is C) Thermal str ns, what is the num D) 6 bar is taken by the ress C) Shear str iform pressure of 3	l layer gated nor shorten ter of gravity [thermal stresses re and plane stres called [ess d) None hber of independ [concrete? [ress d) Bend)) None] sses] ent stress] ing stress
 C) Directing D) Does response of the constraint of the constraint	ly proportional te not depend on the of a beam, which oad B) se are types of n and compressive d bending nich acts in a dire ress B) aded under plane nts? B) 2 of stress does in a stress B) as a Poisson's rat What will be the B) 0	o the neutral lay e distance of lay n is the layer wh Neutral axis ormal stresses? stresses ection perpendio Normal stress e stress conditio C) 3 a reinforcement Compressive st tio of 0.5. If unite volumetric stra 0.20	yer yer from the neutra ich is neither elong C) Cen B) Tensile and D) Compressiv cular to the area is C) Thermal str ns, what is the num D) 6 bar is taken by the ress C) Shear st iform pressure of 3 ain of it?	l layer gated nor shorten ter of gravity D (thermal stresses ve and plane stres called [ess d) None hber of independ [ress d) Bend 00GPa is applied [D) Zero)) None] sses] ent stres] ing stres] to that]

Key:									
1)A	2)B	3)A	4)B	5)A	6)A	7)B	8)A	9)D	10)A
11) C	12) C	13) C	14)D	15)A	16)C	17)C	18)C	19)C	20)B
21) B	22) B	23) C	24)C	25)B	26)C	27)C	28)C	29)D	30)B
31)A	32)B	33)A	34) B	35)A	36) B	37)C	38)B	39)D	40)D

<u>UNIT-II</u>

DIRECT AND BENDING STRESS AND UNSYMETRICAL BENDING

1) A solid shaft of diam A) Td ³	eter D transmits the to B) 3τD	orque equal to C) τD /3	D) τD* 3	[]
to		of external diameter (D)		d) is equ [ual]
A) τ [D ³ -d ³]	B) τ [D ³ -d ³]	C) τ [D ⁴ -d ⁴]	D) τ [D ⁴ –d ⁴]		
3) Polar moment of iner A) [D ³ -d ³]	rtia of a hallow circula B) [D ⁴ –d ⁴]	ar shaft is C) [D ³ –d ³]	D) [D ⁴ –d ⁴]]]
4) The torsional rigidity A) Maximum tw C) Minimum tw	vist in shaft	as the torque required to B) Maximum she D) A twist of one	-	[th of shaft]
5) Polar modulus of sha A) J*R	uft is B) J/R	C)	R/J D)	[) 1/ J]
6) If a shaft is simultar	neously subjected to a	a torque T and a bending	g moment M, the ratio	of maxim	um
shearing stress is given A) 2M/T	by B) M/T	C) 2T/M	D) T/M	[]
maximum stress, then the [hey should have equal	of which is hollow, tra	nsmit equal torque ar C) Diameter D)	-	ual
the shaft are fixed. What	at is the resisting torqu		t section C such that A D) 0.33T	[of]
A) T	B) 0.67T	C) 0.5T	D (0.551		
9) A shaft turns at 150r	pm under a torque of	C) 0.51 150Nm. Power transmitte C) 0.75II kW	,	[]
 9) A shaft turns at 150rg A) 0.15Π kW 10) The outside diameter 	pm under a torque of B) 10П kW ter of a hallow shaft shaft of the same mate	150Nm. Power transmitte C) 0.75II kW is twice its inside diame erial and the same outside	ed is D) 750П l eter. The ratio of its to	[kW] ing]
 9) A shaft turns at 150rg A) 0.15Π kW 10) The outside diametric capacity to that a solid s A) 15/16 11) In a rectangular shaft. 	pm under a torque of B) 10П kW ter of a hallow shaft shaft of the same mate B) 3/4 ft is subjected to torsi	150Nm. Power transmitte C) 0.75II kW is twice its inside diame erial and the same outside	ed is D) 750Π l eter. The ratio of its to e dia is) 1/16 occurs at	[kW prque carryi [] ing]
 9) A shaft turns at 150rg A) 0.15Π kW 10) The outside diametric capacity to that a solid s A) 15/16 11) In a rectangular sha A) Centre E 12) A solid shaft of diametric capacity of the solid shaft of diametric capacity is a solid shaft of diametric capacity of the solid shaft of diametric capacity is a solid shaft of diametric capacity of the solid shaft of the solid solid shaft of the solid so	pm under a torque of B) 10П kW ter of a hallow shaft shaft of the same mate B) 3/4 ft is subjected to torsi 3) corners ameter d carries a tw	 150Nm. Power transmitter C) 0.75II kW is twice its inside diameterial and the same outsider C) 1/2 D on, the maximum shear of 	ed is D) 750Π l eter. The ratio of its to e dia is) 1/16 occurs at e D) middle of larg elops maximum shear s	[kW prque carryi [ger side stress τ. If t]] the
 9) A shaft turns at 150rg A) 0.15Π kW 10) The outside diametric capacity to that a solid s A) 15/16 11) In a rectangular sha A) Centre E 12) A solid shaft of dia shaft is replaced by a h 	pm under a torque of B) 10П kW ter of a hallow shaft shaft of the same mate B) 3/4 ft is subjected to torsi 3) corners ameter d carries a tw	150Nm. Power transmitte C) 0.75II kW is twice its inside diame erial and the same outside C) 1/2 D) on, the maximum shear of C) middle of smaller side isting moment that deve	ed is D) 750Π l eter. The ratio of its to e dia is) 1/16 occurs at e D) middle of larg elops maximum shear s	[kW prque carryi [ger side stress τ. If t]] the
 9) A shaft turns at 150rg A) 0.15Π kW 10) The outside diametric capacity to that a solid s A) 15/16 11) In a rectangular sha A) Centre E 12) A solid shaft of diametric shaft is replaced by a hist stress will be A) 1.607τ 13) If a shaft is rotating 	pm under a torque of B) 10Π kW ter of a hallow shaft shaft of the same mate B) 3/4 ft is subjected to torsi B) corners ameter d carries a tw allow one of outside B) 1.143τ	150Nm. Power transmitte C) 0.75II kW is twice its inside diame erial and the same outside C) 1/2 D) on, the maximum shear of C) middle of smaller side isting moment that deve diameter d and inside diameter d	ed is D) $750\Pi H$ eter. The ratio of its to e dia is) $1/16$ beccurs at D) middle of larg elops maximum shear s ameter d/2, then the m D) 2τ	[kW prque carryi [ger side stress τ. If t aximum sho]] the ear [
 9) A shaft turns at 150rg A) 0.15Π kW 10) The outside diamet capacity to that a solid s A) 15/16 11) In a rectangular sha A) Centre E 12) A solid shaft of dia shaft is replaced by a h stress will be A) 1.607τ 13) If a shaft is rotating by the shaft is 	pm under a torque of B) 10Π kW ter of a hallow shaft shaft of the same mate B) 3/4 ft is subjected to torsi ameter d carries a tw allow one of outside B) 1.143τ g at N revolutions per	150Nm. Power transmitte C) 0.75II kW is twice its inside diame erial and the same outside C) 1/2 D) on, the maximum shear of C) middle of smaller side isting moment that deve diameter d and inside diameter di	ed is D) $750\Pi H$ eter. The ratio of its to e dia is) $1/16$ beccurs at D) middle of larg elops maximum shear s ameter d/2, then the m D) 2τ	[kW prque carryi [ger side stress τ. If t aximum sho]] the ear [

 14) Unsymmetrical bending is the bending caused by loads that A) Lie in a vertical plane B) Lie in a horizontal plane C) Do not lie in a plane containing the principal centroidal axis D) Lie in a plane containing the principal centroidal axis 	[]
14) In a channel section symmetrical about XX axis shear centre lies atA) Centre of the vertical web B) Centre of top flangeC) Centroid of the section E	[)) None]
15) In an I section, symmetrical about XX and YY axes, shear centre lies atA) Centroid of top flange B) Centroid of web C) Centroid of bottom flange	[D)Nor] ne
16) The theory of curved beam was postulated by A) RankineC) CastiglianoD) Winkler-	[Bach]
17) In curved beams the distribution of bending stress A) linearD) HyperboB) ParabolicC) Uniform	[lic]
18) The nature of stress at the inside surface of a crane hook is A) ShearB) TensileC) CompressiveD) None	[]
19) In a closed ring when a small cut is made at the horizontal diameter the maximum stress wiA) DecreaseB) IncreaseC) Remain sameD) Become	-]
20) What is the expression of the bending equation? A) $M/I = \sigma/y = E/R$ B) $M/R = \sigma/y = E/I$ C) $M/y = \sigma/R = E/I$ D) $M/I = \sigma/R = E/y$	[]
21) On bending of a beam, which is the layer which is neither elongated nor shortened?A) Axis of loadB) Neutral axisC) Center of gravityD) Neutral axis	[one]
22) Which stress comes when there is an eccentric load applied?A) Shear stressB) Bending stressC) Tensile stressD) Thermal stress	[stress]
23) Stress in a beam due to simple bending isA) Directly proportional B) Inversely proportional C) Curvilinearly related	[D) No] ne
24) The maximum negative bending moment in fixed beam carrying udl occurs atA) Mid spanB) 1/3 of the spanC) SupportsD) Half of the span]
25) A fixed beam of the uniform section is carrying a point load at the centre, if the moment of	inertia o	f
A) IncreaseB) Remains constantC) DecreaseD) Change their direction	l ction]
26. A propped cantilever beam carrying total load "W" distributed evenly over its entire length	-	Э г
the vertical force required in the prop.	Ĺ]
A) 3/4 W B) W C) 5/8 W D) 3/8 W		
27. A solid shaft of circular in section is subjected to torque which produces maximum shear st	ress in a	L
shaft. Calculate the diameter of the shaft.	[1
A) $(16T/\pi f)^{3/2}$ B) $(16f/\pi T)^{1/2}$ C) $(16f/\pi)^{1/2}$ D) $(\pi T/16f)^{1/2}$	L	-
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	A) Integrated sharts	B) Composite sh	alls () D	illerential shal	ts	D) Combine	ed shafts	j
9	torque occ	urs along with maxi	imum shea	r stress due to	combine	ed bending and	torsior	1.
	A) Equipment	B) Coaxia	.1	C) Biaxial		D) Lateral	[
0. W	Then a shaft is subjected	d to pure twisting th	nen the typ	e of stress dev	eloped is	5	[
	A) Bending	B) Axial	C) SI	near	D) No	ormal		
1. T	he torque which produ	ces unit twist per u	nit length	is			ſ	
	A) Torsional rugosi				ity D) To	rsional mean	radius	
2. In	simply supported bear A) Bending					ompressive	[
3. Tł	ne maximum		1	1	~ 11		[
	A) Tensile	B) Compressive	C) Sh	iear	D) Be	nding		
4. Tł	ne stress is directly pro A) E	portional to B) u	— C) y		D) R		[
5. At	t the extreme fibre, ber A) Minimum	0		C) Constant		D) Maximu	[m	
6. Tł	ne curvature of a beam A) EI/M	is equal to B) M/E		C) M/EI		D) E/MI	[
7. Sk	cin stress is also called	as					[
	A) Shear stress	B) Bendir	ng stress	C) Lateral st	tress	D) Tempera	ture stre	25
8	A) modulus of resilie		•	C) resilienc	e	D) proof res	[ilience	
9 In	cantilever beams, there A) Compressive	e is stress a B) Tensile		tral axis. C) Tempera	ture	D) Shear		
.0 Tł	ne strength of beams do	epend merely on					[
	A) Modulus section	B) Momer	nt of inerti	a C) Flexural	rigidity I	D) Moment of	resistar	10

<u>UNIT –III</u> INTRODUCTION

 For the validity of principle of s A) linear-elastic B) non-lin 		terials should be C) Non-linear-		r? [] e ar- inelastic
 2. If in planar system, X parts/mendeterminacy is:- A) Y < 3X B) Y 		<i>C</i>) Y = 3X	ces, then condition for D) none	statically []
 3. If Y > 3X (X and Y are from the A) Statically indeterminate C) Can't say 	B) Statically d			[]
4. If in a planar system, only 2 reaA) Essentially unstable	action forces are B) Essentially	-	•	[] D) None
5. If all the reactions acting on a pA) Can't say	blanar system are B) Essentially		ature, then the system sentially unstable	is:- [] D) none
6. If 4 reactions are acting on a beA) Unstable & indeterminate	eam, then the sys B) Stable & in		C) Stable & determina	[] te D) Can't say
7. If a system has more equationsA) Improperly constrained	of equilibrium the B) Partially c e			[] D) Solvable
 8. How many cases out of the folloparallel forces, concurrent force A) 1 B) 2 	0 1 1	•		[]
 9. If a structure has total 10 joints equations should be concurrent A) 7 		ld be the minim	,	ich equilibrium []
10. If a structure has 2j – r no. of mA) stableB) unstable			D) depends upon magi	[] nitude of load
11. Which of the following materiaA) Wooden strutsB) M	al is not used in n etal bars	naking trusses? C) Channel	D) Concrete	[]
12. The space between adjacent ber A) Purlins B) B a		is called:- C) Knee	D) Braces	[]
13. In a bridge truss, what is the seA) Stringers to floor beams toC) Side trusses to stringers to fl	side trusses	B) Floor beams	to stringers to side tru to floor beams to strin	
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14. What is the function					[]
A) To resist lateraC) To provide ad			ist horizontal forces ow thermal expansion			
15. Pratt, Howe and w A) 100 ft	varren trusses are use B) 150 ft		much span length?) 200 ft	D) 250 ft	[]
16. In a truss it is assu A) Rough pins	umes that the membe B) Smooth p	•) Either of them	D) Neither of	[them]
17. There is no bendin A) Assumptions 1	•	e to:- C) Mater	ials used	D) Neither of	[them]
18. How many equilibA) 1	orium equations do w B) 2	e need to solve gen C) 3	erally on each joint	of a truss? D) 4	[]
19. If a member of a t the joints?	truss is in compressio	on, then what will b	e the direction of fo	rce that it will	apply to [o]
A) Outward	B) Inward	C) Depen	ids on case D) No	force will be	there	
20. If a member of a t joints?	truss is in tension, the	en what will be the	direction of force th	at it will apply	to the]
A) Outward	B) Inward	C) Depen	ids on case D) No	force will be t	here	
21. What should be id A) Draw fbd of ea	ich joint	B) Draw fbd of c	overall truss		[]
C) Identify zero f			ternal reaction force	:5	r	1
22. Which of the folloa) FG, HI, HJ	b) HI, HJ, AE	c) HI, HJ, HE	d) HI, HJ, FH		L	Ţ
23. What should be th	e angle (in degrees) i be a zero force memb	•	(part of a bigger sys	stem) if both o	f the	1
a) 22.5	b) 45	c) 67.5	d) 90		L	J
24. What will the mag		,			[]
a) 0	b) 1	c) 2	d) 3		r	1
25. What will the mag a) 0	b) 1	c) 2	d) 3		L	J
26. What is total no. (a) 7	of zero force member b) 8	rs in the above give c) 9	n system? d) 10		[]
27. What is the except	,	,		be more impo	rtant th:	an
resistance to bendi	ing and shear force in	n beams?		••••••••••••••••••••••••••••••••••••••	[]
a) Expansive axial c) Bending momen		b) Compressive d) Bending mom				
28. What will be the s	-		1		[]
a) 10 Mechanics of solids (18	b) -10 8CE0111)	c) 5	d) -5		Page 1	17

29. What will be the	shape of SFD in the er	ntire beam?			[1
a) Straight line			h point of discontinuiti	es	L	1
c) Curve		d) Curve with point	-			
	s of discontinuities wil				[]
a) 1	b) 2	c) 3	d) 4			
		. 1 0			r	1
	shape of BMD in the e		point of discontinuition		L]
a) Straight line c) Curve		d) Straight line with	point of discontinuities			
c) curve		u) Straight line wit	in varying slope			
32. How many times	would slope of BMD	change?		[1	
a) 1	b) 2	c) 3	d) 4	L		
33. What will be the	starting value of SFD?	2		[]	
a) 25	b) -25	c) 15	d) -15			
					r	
	e shape of SFD in the e				L	
a) Straight line c) Curve	. 0	ine with points of dis				
c) Curve	u) straight in	le with points of disce	ontinuities parallel to x a	XIS		
35. How many point	ts of discontinuities wi	ll be there in the SFD	?		[1
a) 1	b) 2	c) 3	d) 4		L	1
36. What will be the	starting value of BMD)?		[]	
a) 20	b) -20	c) 10	d) -10			
07 WH - 111 - 1					r	
	shape of BMD in the e				L]
a) Straight line c) Curve		-	point of discontinuities curve and straight line			
c) Curve		u) Complitation of	curve and straight line			
38. How many times	s would slope of BMD	change?		ſ	1	
a) 1	b) 2	c) 3	d) infinite	L		
-	uestion $R = 4$ then the t	truss are:-			[]
a) Statically deter	rminate	b) Statically indete	rminate			
c) Stable		d) Unstable				
40 A surface struct	ura haa			г	1	
a) Small thickne		b) Large thic	kness	L]	
c) Moderate thick		d) Arbit thickness	-KIIC55			
c) moderate uner		a) Thore unexhests				

Key:

1) D	2)C	3)A	4)A	5)C	6)D	7)B	8)B	9)D	10)C
11) D	12) B	13) A	14)C	15)C	16)B	17)A	18)B	19)A	20)B
21) C	22) A	23) D	24)A	25)A	26)C	27)B	28)B	29)B	30)C
31)D	32)C	33)A	34)B	35)B	36)D	37)D	38)D	39)B	40)D

UNIT-IV

ANALYSIS OF FIXED BEAMS & CONTINUOUS BEAMS

	Rotation at the A) L/2	e fixed end B) L/4	C) Zero	D) none	[]
	Net moment a A) Zero	t the support B) double	C) half	D) none	[]
	Bending Mom A) Directly pr C) Equal		to shear force B) Indirectly D) all the abo		[]
	Degree of free A) Zero	edom for fixed B) 1	end condition l C) 2	Deam D) 3	[]
5.		•		entire span. The joints of from the centre D) $1/4\sqrt{3}$		e will occur]
6.		tructural members (tructural members) tructural members (tructural members) axi	-	tly subjected to twisting moment	[D) none of the] above
7.	The moment of	listribution me	thod is best sui	ted for	[]
	A) Indetermin	ate pin jointed	truss B) Rig	gid frames		
	C) Space fram	ies	D) Tri	ussed beam		
	considering A) The equilit	ction method, t prium of the joi prium of the str	int B) Th	tations at various joints ne rigidity of the joint one	s are determined	l by]
9.	Additional sp		-	d end of a continuous b ro moment of inertia	beam is replaced [D) None of the]
	B) Simply su	beam carrying pported beam c beam, carryin	carrying a conc	l load W at its free end entrated load W at mid listributed load over spa	-span is WL3/4] 8EI
			supported bean cal axis B) Sum	n is proportional to n of the forces]	`]
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C) Sum of the transverse forces D) Algebraic sum of the transverse	se forces	of the se	ction
12. In a loaded beam, the point of contra-flexure occurs at a section where A) Bending moment is minimum B) Bending moment is zero or ch C) Bending moment is maximum D) Shearing force is maximum] gn	
13. The shape of the bending moment diagram over the length of a beam, or increasing load, is alwaysA) Linear B) Parabolic C) Cubical D) Circular	carrying [a uniform]	ıly
14. The shape of the bending moment diagram over the length of a beam, of distributed load is alwaysA) Linear B)Parabolic C) Cubical D) Circular	carrying [a uniform]	ıly
15. Shear force for a cantilever carrying a uniformly distributed load over B) Triangle B) Rectangle C)Parabola D) Cubic parabola	its lengtl [n, is]	
 16. For a beam having fixed ends, the unknown element of the reactions, A) Horizontal components at either end B) Vertical components a C) Horizontal component at one end and vertical component at the other D) Horizontal and vertical components at both the ends 	t either e] end	
17. If the shear force along a section of a beam is zero, the bending mom A) Zero B) Maximum C) Minimum D) Average of maximum-mini		e section [is
18. The moment diagram for a cantilever carrying a concentrated load at A) Triangle B)Rectangle C)ParabolaD) Cubiparabola	its free	end, will b	De
19. The bending moment is maximum on a section where shearing forceA) Is maximumB) Is minimumC) Is equalD) Changes]	
20. For a simply supported beam with a central load, the bending momA) Least at the centreB) Least at the supportsC) Maximum at the supportsD) Maximum at the centre	ent is	[]	
20In a continuous bending moment curve the point where it changes signA) Point of inflexionB) Point of contra flexureC) Point of virtual hingeD) All the above	n, is calle [ed]	
21) The max deflection of a simply supported beam of length L with a co is A)WL ² /48EI B)W ² L/24EI C)WL3/48EI D)WL ² /8EI	entral loa [nd W,]	
 22) A simply supported beam carries two equal concentrated loads W at either support. The maximum bending moment A) WL/3 B)WL/4 C)5WL/4 D)3WL/12 	distance [s L/3 from]	n
23) A cantilever of length is subjected to a bending moment at its free en flexural rigidity of the section, the deflection of the free end, is A)ML/EI B)ML/2EI C)ML ² /2EI D)ML ² /3EI	nd. If EI : [is the]	
24) In a fixed beam, at the fixed endsA) Slope is zero and deflection is maximum	[]	
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B) Slope is maximum and dC) Both Slope and deflectionC) Slope and deflection are	on are maximun				
25) A beam fixed at both endsbending moment at the centreA) 10KN-m B) 30KN-m	of the beam is	of 20KN/m over th D) 90KN-m	he entire span c [of 6 m. Th]	e
26) A beam ABC is simply su		-	-		with
UDL the maximum number o A) 0 B)1 C) 2 D)3	f contra-flexure	e points in the beam	n will be equal	to]	
27. The rate of change of bendiA) Shear force B)Horizonta	0 1		[) none]	
28The units of bending momen (A) KN		\mathbf{D} \mathbf{U} \mathbf{U} \mathbf{U}	[]	
A) KN-m B) KN- m^2	C) KN/m	D) KN/m ²			
29 A fixed beam AB 6 m long of at A and B are	carries a vertica	l load 90 KN at 2m	from A. The f	ixed end r]	noments
A) 40 KN-m, 80 KN-m	B) 40 KN-m				
C) 80 KN-m,40 KN-m	D) 120 KN-r	n,80 KIN-m			
 30. In a fixed beam is subjected at A) L/2 B) At two fix D) 0.667 L from each of the 	ked supports	C) 0.21 L from each	[]	l occur
31. The point of contra flexure in	n a fixed beam	carrying UDL will	l occur at a dist	ance	- from
the ends			[
A) $L/\sqrt{3}$ B) $L/\sqrt{2}$	C) L/3√2	D)L/2√3			
33. A beam having more than toA) Fixed B) Continuo			[y Supported]	
34. Fixed beam is moreA) Stable B) Stronger	C) S	tiffer	[D) All]	
35. At the point of contra flexurA) Maximum B) Minimum		s D) Negative	[]	
36. At the maximum deflection A) Maximum B) Minimum	-	D) Negative	[]	
37. A simply supported beam of	f span L carries	a uniformly distrib	outed load W. 7	The maxin	num
bending moment M is A) $\frac{WL}{2}$ B) $\frac{WL}{4}$ C) $\frac{WL}{8}$ D) $\frac{WL}{12}$	-		[]	
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- - A) $\frac{WL}{2}$ B) $\frac{WL}{4}$ C) $\frac{WL}{8}$ D) $\frac{WL}{12}$
- 39. For a simply supported beam with a central load, the bending moment is []A) Least at the center. B) Least at the supports. C) Maximum at the supports. D) Maximum at the center
- 40. The deflection of any rectangular beam simply supported is []
 A) Directly proportional to its weight B) Inversely proportional to its weight
 C) Inversely proportional to the cube of its depth D) All the above

C) Inversely proportional to the cube of its depth D) All the above

Key: 1)C 4)A 7)B 8)A 9)A 10)D 2)A 3)B 5)B 6)A 11) D 12) B 13) C 14)B 17)B 19)D 20)D 15)B 16)D 18)A 21) D 22) C 23) A 24)D 25)D 26)B 27)B 28)A 29)A 30)C 31)C 32)A 33)B 34)D 36)C 38)B 40)D 35)C 37)C 39)D

<u>UNIT- V</u>

SLOPE DEFLECTION METHOD& MOMENT DISTRIBUTION METHOD

1.	The number of independent equations to be satisfied for static equilibrium of a plies	ane stru	-			
	A) 1 B) 2 C) 3 D) 6	L]			
2.	In the slope deflection equations, the deformations are considered to be caused by	7				
		[]			
	i. Bending moment					
	ii. Shear force					
	iii. Axial force					
	The correct answer is					
	A) Only (i) B) (i) and (ii) C) (ii) and (iii) D) (i), (ii) and (iii)					
3.	The fixed end moment for continuous beam subjected to UDL	[]			
	A) $\frac{wl^2}{12}$ B) $\frac{wl^3}{12}$ C) $\frac{Wl}{8}$ D) $\frac{wab^2}{l^2}$					
4.	The fixed end moment for continuous beam subjected to central point load	[]			
	A) $\frac{wl^2}{12}$ B) $\frac{wl^3}{12}$ C) $\frac{Wl}{8}$ D) $\frac{wab^2}{r^2}$					
5.	The fixed end moment for continuous beam subjected to eccentrically point load					
	A) $\frac{wl^2}{12}$ B) $\frac{wl^5}{12}$ C) $\frac{Wl}{8}$ D) $\frac{wab^2}{l^2}$	г	1			
		L]			
6.	Slope deflection equation MAB =	[]			
	A) FAB+ $\frac{2EI}{l}(2\theta_A + \theta_B)$ B) FAB - $\frac{2EI}{l}(2\theta_A + \theta_B)$					
	C) FBA+ $\frac{2EI}{l}(2\theta_B + \theta_A)$ D) FBA+ $\frac{2EI}{l}(2\theta_A + \theta_B)$					
7.	A continuous beam AB subjected to UDL of 20 kN/m then fixed end moment FA	B is				
	A) 40 kN-m B) 120 kN-m C) 60 kN-m D) 180 kN-m	[]			
8.	A continuous beam AB subjected to central point load of 60 kN then fixed end m	oment	FAB is			
		[]			
	A) 40 kN-m B) 45 kN-m C) 60 kN-m D) 80 kN-m					
9.	Frames may sway due to	[]			
	A) Horizontal force & unsymmetry B) horizontal force only					
	C) unsymmetry of columns D) all the above					
10	. A beam subjected to UDL then bending moment diagram is in shape					
	A) Triangle B) rectangle C) parabola D) cubic	[]			
11.	A beam subjected to point then bending moment diagram is inshape	_	_			
	A) Triangle B) rectangle C) parabola D) cubic	[]			
12	A beam subjected to UVL then bending moment diagram is inshape	r	-			
10	A) Triangle B) rectangle C) parabola D) cubic	[]			
13	13. The develop method for slope deflection method is []					
	A) Flexibility method B) Kani's method					
	C) Stiffness matrix method D) moment distribution method					

QUESTION B.	ANK 2	019
14. In the displacement method of structural analysis, the basic unknowns areA) DisplacementsB) force	[]
C) Displacements and forces D) none of the above		
15. In the slope deflection equations, the deformations are considered to be caused b	v	
i) B.M. ii) S.F.iii) axial force	5	
The correct answer is:	ſ	1
A) Only I B)i and ii C) ii and iii D)all three	L	J
16. Bending moment at any section in a conjugate beam gives in the actual beam		
A) Slope B) curvature C) deflection D) B.M.	[]
17. The statically indeterminate structures can be solved by	L F	ı ı
A) Using equations of statics alone B) Equations of compatibility alone	L]
C) Ignoring all deformations and assuming the structure is rigid	•1•,	
D)Using the equations of statics and necessary number of equations of compatibute	llity	_
18. A beam is completely analysed,	L]
A) Support reactions are determined B) Shear and moment diagrams are	found	
C) The moment of inertia is uniform throughout the length		
D) All of the above		
19. A bending moment may be defined asA) Arithmetic sum of the moments of all the forces on either side of sectionB) Arithmetic sum of the forces on either side of sectionC) Algebraic sum of the moments of all the forces on either side of sectionD) None of these]]
20. At either end of a plane frame, maximum number of possible transverse shear for A) One B) two C) threeD) four	orces, a	re
21.In moment distribution method, the sum of distribution factors of all the members mee	ting at a	any
joint is always	[]
A) ZeroB) less than 1C) 1D) greater than 122. The carryover factor in a prismatic member whose far end is fixed is	[]
A) 0 B) $\frac{1}{2}$ C) $\frac{3}{4}$ D) 1	L	1
23.Carry over factor =	[]
$A)\frac{M}{\theta_A} \qquad B)\frac{M}{M} \qquad C)\frac{M'}{M} \qquad D)\frac{M}{M'}$		
24. Stiffness K=	[]
A) $\frac{\theta_A}{\theta_A}$ B) $\frac{\theta_A}{M}$ C) $\frac{M'}{M}$ D) $\frac{M}{M^l}$		
25. Distribution factor =	[]
A) B) $\frac{\Sigma \kappa}{\kappa}$ C) $\frac{M}{\Sigma \kappa}$ D) $\frac{\kappa}{\Sigma \kappa}$		
26.If the far end is fixed then stiffness K=	[]
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A) $\frac{4EI}{L}$	B) $\frac{3EI}{L}$	C) $\frac{2EI}{L}$	D) $\frac{EI}{L}$			
27.Which of the	following meth	ods of structur	al analysis is	a displacement n	nethod []
A) Moment	distribution met	hod	B) colum	n analogy method	1	
C) Three m	oment equation		D) none	of the above		
28.In the displac	ement method	of structural an	alysis, the ba	sic unknowns are	. []	
A) Displace	ements	B) force				
C) Displace 29.The moment	ements and force distribution me			oservers-2013)	[]	
A) Indeterm	ninate pin jointe	d truss B) F	Rigid frames			
B) Slop 31.The statical C) Using ec	oment at any sec be B) cu ly indeterminate	ction in a conju urvature e structures can cs alone B) E	C) deflec be solved by Equations of c	ives in the actual l tion D) B.M. 7: compatibility alor	[]]
D) Using th 32.The simultan D) Moment	e equations of s eous equations	tatics and nece of slope deflec thod B) C	essary numbe	r of equations of c can be solved by i formation method	iteration in: [
		,		nd is hinged is (A	EE-2008)	[]
A) 0B) 1/2	C) 3/4	D) 1	c		•. •	•.•
	-		-	ic beam through a (AEE-1996, 2004	-	
GENCO-15			, 10 B1 (011 0 J)	(,	
E) 3EI/L	B) 4EI/L	C)2EI/L	D)EI/L]]
35.The moment	required to rota	te the near end	of a prismat	ic beam through a	a unit angle w	vithout
translation, the f	-		-	(TSPSC-A	-]
A)EI/L	B) 2EI/L	C) 3	EI/L	D)4EI/L		
	s flexural rigidit sternal moment	•	-	eam. of a prismatic bea	am without tr	anslation
F) M/2 in the C) M in opp	he same direction	on as M B) M/2 D) (2 in the oppos	r end is (AEE-20 site direction as N	1]
without any rota	-	ansverse reacti	ons at A or E	s given a transve B due to displacen	-	
38.Moment-dist G) Hardy C	ribution method ross B) G	l was suggested A. Maney	l by C) Gaspe	r Kani D) N y distributed load	[None of these W The maxi]
bending mome				, and the total]
A) $\frac{WL}{2}$	B) $\frac{W}{4}$	$\frac{L}{C}$ C) $\frac{W}{C}$		12	L	
		-	es a concenti	rated load W at its	s mid span. T	he
maximum bene A) $\frac{WL}{2}$	ding moment M B) $\frac{W}{4}$		C) $\frac{WL}{8}$	D) $\frac{WL}{12}$	[]
-	-					

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Key: 1)C 2)A 11) A 12)D			/	9)D 19)A	/
21)C 22)B 31) A 32)A		26)A 36)B	,	29)B 39)C	,